



ORIGINAL PAPER

A NEW APPROACH FOR THE PREDICTION OF BRITTLENESS INDEX BASED ON CHEMICAL PROPERTIES OF BASALTIC ROCKSCandan BILEN ¹⁾*, Selman ER ²⁾, Atiye TUĞRUL ³⁾ and Murat YILMAZ ³⁾¹⁾ Department of Geological Engineering, Zonguldak Bulent Ecevit University, Zonguldak, Turkey²⁾ Department of Geography, Inonu University, Malatya, Turkey³⁾ Department of Geological Engineering, Istanbul University-Cerrahpasa, Istanbul, Turkey*Corresponding author's e-mail: candanalpTekin@gmail.com**ARTICLE INFO****Article history:**

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ABSTRACT

Rock brittleness is one of the most important issues in rock mechanics. There is not yet an available method for defining or measuring directly the rock brittleness. The aim of this study is to suggest a new chemical index parameter for the prediction of basaltic rocks' brittleness. In the order of that abovementioned purpose, a total of 23 basaltic rock samples were collected from different region of Turkey. Samples were initially tested to determine their chemical properties. Then, mechanical tests were carried out to define the brittleness indices (B1, B2, and B3) for each corresponding sample. Finally, relations between parameters obtained from test results and brittleness indices were examined with regression analysis. According to the results obtained, a new chemical parameter (CP) was proposed for predicting brittleness via major oxide element components of basaltic rocks. It was found out that, B1 and B2 are not reliable parameters for predicting the different properties, however; B3 and CP can be employed as good criteria for predicting the different properties of basaltic rocks (especially in terms of chemical and mechanical properties).

1. INTRODUCTION

The brittleness is generally contemplated as one of the most important mechanical properties of rock. Rock brittleness is one of the most popular research areas in rock mechanics and this is because some rocks show brittle behavior when exposed to different type of loads. Brittleness characteristics as important geotechnical parameters which are used in rock engineering projects such as tunneling and underground openings, dam foundations, drilling, and slope stability analysis (Aligholi et al., 2017). The brittleness index (BI) is a term that is usually used to quantify the brittleness of rock mass. The uniaxial compressive strength (UCS) and Brazilian tensile strength (BTS) are two main geomechanical properties of rock that can be used for the brittleness index (BI) prediction (Zhang et al., 2016).

An excessive number of different measures of rock brittleness can be found in the literature. For example, Yagiz (2009) have tried to evaluate the rock brittleness with punch penetration test and he has tried to predict rock brittleness in terms of a function based on uniaxial compressive strength, Brazilian tensile strength and density of rock. Although Yagiz (2009) stated the fact that "there is no confident way to measure rock brittleness directly", same author has suggested the punch penetration test in the order of direct measurement of rock toughness and brittleness

(Yagiz, 2006). In this context, Yagiz and Karahan (2015) have employed the same data of rock brittleness derived from punch penetration test in order to understand TBM (tunnel boring machine) performance. Although many researchers have tried to define rock brittleness indirectly, there is no yet an agreement to describe or measure it (Yagiz, 2009). This abovementioned shortage of rock brittleness description is also explained by Kaunda and Asbury (2016). According to Kaunda and Asbury (2016), strength based methods to describe the rock brittleness under various load conditions and anisotropies are not sufficient and they seem to fall short in describing rock brittleness. In their study of Kaunda and Asbury (2016) have tried to predict rock brittleness by employing the data based on P- and S-wave velocities, elastic properties and rock type. Some other researchers like Guo et al. (2012) however tried to describe rock brittleness based on mineralogy and porosity in shale (non-strength based rock physics). In addition to these abovementioned attempts to describe rock brittleness, Yagiz and Gokceoglu (2010) have employed nonlinear regression models and fuzzy logic to predict rock brittleness. In this context, a summary of literature descriptions of rock brittleness is provided in Table 1 (adapted from the study of Zhang et al. (2021)).

Table 1 Summary of literature description of rock brittleness (Adapted from the study of Zhang et al., 2021).

Equation No	Suggested method for the description of rock brittleness	Note*	Reference
[1]	$b_1 = \sigma_c/\sigma_t$	*i	Hucka and Das (1974)
[2]	$b_2 = \sigma_c\sigma_t/2$	*i	Altindag (2003)
[3]	$b_3 = (\sigma_c\sigma_t/2)^{0.5}$	*i	Altindag (2003)
[4]	$b_4 = (\sigma_c - \sigma_t)/(\sigma_c + \sigma_t)$	*i	Hucka and Das (1974)
[5]	$b_5 = \sigma_c/\sigma_t = 8\sigma_c/\sigma_{ci} = 8/K$	*i	Wang et al. (2014)
[6]	$b_6 = (\sigma_c - \sigma_t)/(\sigma_c + \sigma_t) = (8-K)/(8+K)$	*i	Wang et al. (2014)
[7]	$b_7 = (\tau_p - \tau_r)/\tau_p$	*i	Bishop (1967)
[8]	$b_8 = \epsilon_r/\epsilon_t$	*ii	Hucka and Das (1974)
[9]	$b_9 = \epsilon_{ini}/\epsilon_t$	*ii	
[10]	$b_{10} = (\epsilon_p - \epsilon_r)/\epsilon_p$	*ii	Bishop (1967)
[11]	$b_{11} = (\epsilon^p_f - \epsilon^p_c)/\epsilon^p_c$	*ii	Hajiabdolmajid and Kaiser (2003)
[12]	$b_{12} = \epsilon_{11} \times 100\%$	*ii	Andreev (1995)
[13]	$b_{13} = (\tau_p - \tau_r)/\tau_p \cdot \lg k_{ac} /10$	*iii	Meng et al. (2015)
[14]	$b_{14} = (\sigma_p - \sigma_r)/(\epsilon_r - \epsilon_p) + [(\sigma_p - \sigma_r)(\epsilon_r - \epsilon_p)]/(\sigma_p \epsilon_p)$	*iii	Xia et al. (2016)
[15]	$b_{15} = [(\sigma_p - \sigma_i)/\sigma_p]/(\epsilon_p - \epsilon_i) + [(\sigma_p - \sigma_r)/\sigma_p]/(\epsilon_r - \epsilon_p)$	*iii	Chen et al. (2019a)
[16]	$b_{16} = W_r/W_t$	*iv	Hucka and Das (1974)
[17]	$b_{17} = dW_r/dW_e$	*iv	Tarasov and Potvin (2013)
[18]	$b_{18} = dW_d/dW_e$	*iv	Tarasov and Potvin (2013)
[19]	$b_{19} = (dW_f + dW_d)/(dW_{ue} + dW_d)$	*iv	Ai et al. (2016)
[20]	$b_{20} = dW_x/(dW_{ue} + dW_d)$	*iv	Ai et al. (2016)
[21]	$b_{21} = E/\nu$	*v	Luan et al. (2014)
[22]	$b_{22} = (E_n + \nu_n)/2$	*v	Rickman et al. (2008)
[23]	$b_{23} = E/\lambda$	*v	Chen et al. (2014)
[24]	$b_{24} = (\lambda + 2G)/\lambda$	*v	Guo et al. (2012)
[25]	$b_{25} = (3K - 5\lambda)/\lambda$	*v	Huang et al. (2015)
[26]	$b_{26} = Ha/Kc$	*vi	Lawn and Marshall (1979)
[27]	$b_{27} = Ha \cdot E/Kc^2$	*vi	Quinn and Quinn (1997)
[28]	$b_{28} = (H_\mu - H_m)/c$	*vi	Hucka and Das (1974)
[29]	$b_{29} = q\sigma_c$	*vii	Protodyakonov (1962)
[30]	$b_{30} = S_{20}$	*vii	Gong and Zhao (2007)
[31]	$b_{31} = F_{max}/P$	*viii	Yagiz (2009)
[32]	$b_{32} = P_{dec}/P_{inc}$	*viii	Copur et al. (2003)
[33]	$b_{33} = K_b, K_s P/h^2 = S_t - K_b P$	*ix	Reichmuth (1968)
[34]	$b_{34} = \sigma_c/\sigma_{c-NC}$	*x	Ingram and Urai (1999)
[35]	$b_{35} = OCR^b, OCR = \sigma_{vmax}/\sigma_v$	*x	Nygård et al. (2006)
[36]	$b_{36} = W_q/W_t$	*xi	Jarvie et al. (2007)
[37]	$b_{37} = (W_q + W_c)/W_t$	*xi	Jin et al. (2014)
[38]	$b_{38} = -1.8748\emptyset + 0.9679$	*xii	Jin et al. (2014)
[39]	$b_{39} = \sin \theta$	*xiii	Hucka and Das (1974)
[40]	$b_{40} = 45^\circ + \theta/2$	*xiii	Hucka and Das (1974)

*Note that:

- i. σ_c and σ_t are the uniaxial compression strength and tensile strength; τ_p and τ_r are the peak strength and residual strength; σ_{ci} and K are the initiation stress and initiation stress level,
- ii. ϵ_t and ϵ_r are the total strain and reversible strain at failure; ϵ_{ini} is the strain at onset of fracture initiation (dilatancy); ϵ_p and ϵ_r are peak and residual strain; ϵ_{pc} and ϵ_{pf} are the plastic strain necessary for cohesion loss and frictional strengthening; ϵ_{11} is the absolute irreversible longitudinal strain at failure,
- iii. τ_p and τ_r are the peak strength and residual strength, k_{ac} is the slope of the post-peak stress drop; σ_i , σ_p and σ_r are the initiation stress, peak strength and residual strength, ϵ_i , ϵ_p and ϵ_r are initiation strain, peak strain and residual strain,
- iv. W_r and W_t are the recoverable energy and total energy at failure; dW_r , dW_e and dW_d are the post-peak rupture energy, the converted elastic energy and the released energy; dW_f and dW_d are the post-peak rupture energy and the pre-peak dissipation energy, and dW_{ue} and dW_x are the unloading elastic energy and the extra energy required or the excess energy released,
- v. E and ν are the Young's modulus and Poisson's ratio; E_n and ν_n are the normalized Young's modulus and Poisson's ratio; λ and G are the first and second Lamé parameters; K is the bulk modulus,
- vi. Ha is the hardness, Kc is the fracture toughness, E is the elastic modulus; H_μ is the micro-indentation hardness, H_m is the macro-indentation hardness, and c is a constant,
- vii. q is the percentage of fines (below 28 mesh) obtained from Protodyakonov's impact test; S_{20} is the percentage of the fines less than 11.2 mm,
- viii. F_{max} is the maximum applied force, P is the penetration depth at the maximum force; P_{inc} and P_{dec} are the average increment and decrement of forces,
- ix. K_b is the relative brittleness index, K_s is the shape factor, P is the applied load at failure, h is the distance between loading points, and S_t is the tensile strength,
- x. σ_{c-NC} is the uniaxial compression strength of a normally consolidated rock in non-overpressured areas; OCR is the over-consolidation ratio, b is the empirical constant, σ_{vmax} and σ_v are maximum effective vertical stress experienced and the current effective vertical stress,
- xi. W_q is the weight of quartz, W_c is the carbonate mineral weight, and W_t is the total mineral weight,
- xii. \emptyset is the neutron porosity,
- xiii. θ is the inner friction angle determined from Mohr's envelope at $\sigma_{vn} = 0$



Fig. 1 Map showing the sampling locations of the basaltic rocks tested in the present work.

Although Zhang et al. (2021) have included 40 different brittleness index approach, Meng et al. (2021) have categorized 11 different groups of brittleness indices (a total of 80 brittleness indices) in their review (Table A1 from Meng et al. (2021)). Mikaeil et. al. (2013) founded a mathematical correlation between production rate in ornamental stones sawing and rock brittleness index. According to Mikaeil et. al. (2013), the production rate decreases with the increase in BI and this fact can be used as a parameter for predicting the production rate for all type of ornamental stones. Some brittleness index definitions derived from stress-strain curves were introduced and used in the literatures (Aubertin et al., 1994; Ribacchi, 2000; HajiabdoImajid and Kaiser, 2003; Tarasov and Potvin, 2013; Meng et al., 2015, Xia et al., 2017; Zhou et al., 2018). Also some researches founded relationships between brittleness and drillability, borability, the specific energy in rock cutting, shore hardness (Kahraman, 2002; Altindag 2000, 2002, 2003; Kahraman and Altindag, 2004; Altindag and Guney, 2010). According to Aligholi (2017), there is a systematic trend between dry density, P-wave velocity, rebound hardness and brittleness value of the studied igneous rocks with medium hardness. The brittleness characteristics of rocks affect their mechanical performance, drillability, cuttability, and machine performance (Aligholi, 2017).

The aim of this study is to evaluate brittleness properties of basaltic rock samples collected from different regions of Turkey and to predict brittleness behavior based on chemical parameter (CP) in terms of their major oxide element contents. The brittleness indices, i.e. B1, B2 and B3 were calculated by employing the uniaxial compressive strength (UCS) and Brazilian tensile strength (BTS) of the rocks. In addition, a new chemical parameter (CP) was

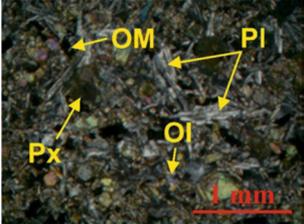
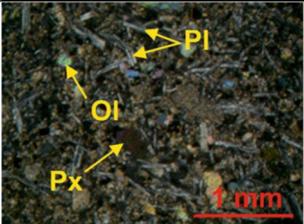
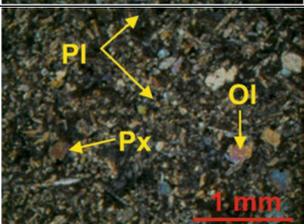
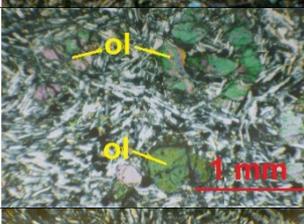
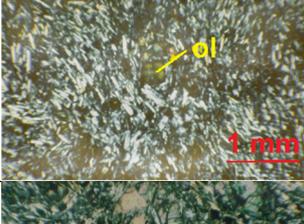
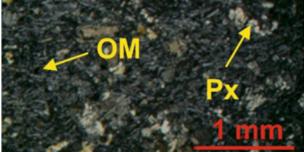
proposed to be in relationship with these abovementioned brittleness index values, i.e. B1, B2, and B3. Corresponding calculation of CP is performed with the consideration of chemical properties (Al_2O_3 , Fe_2O_3 , MgO, SiO_2 values) of the rocks. The obtained results were compared with simple regression method.

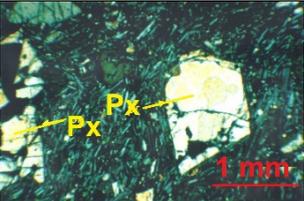
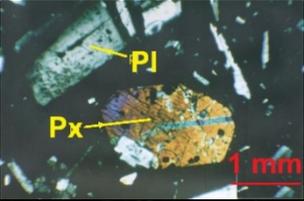
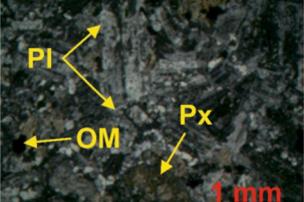
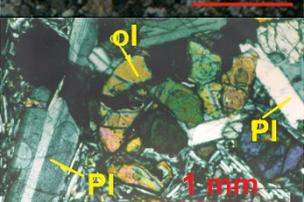
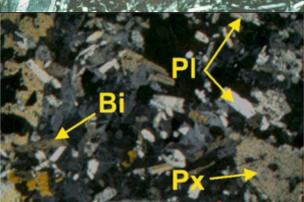
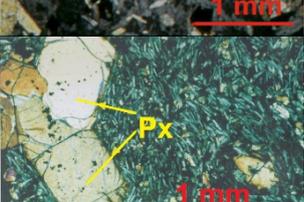
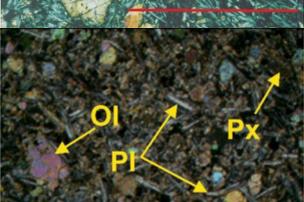
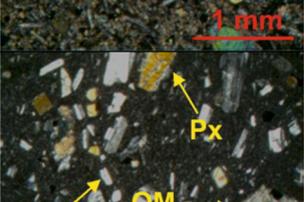
2. MATERIAL AND METHOD

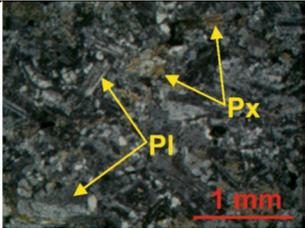
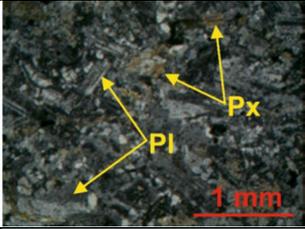
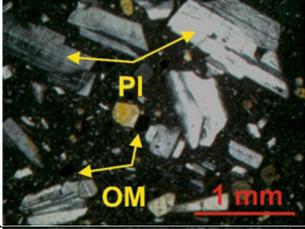
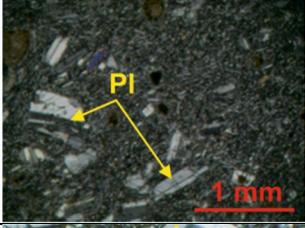
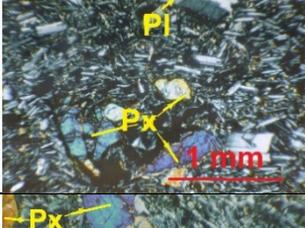
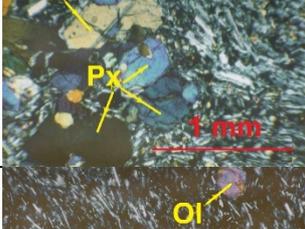
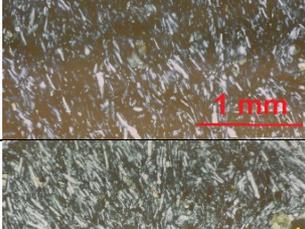
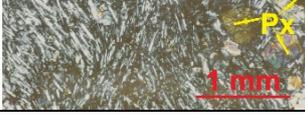
2.1. PETROGRAPHIC AND CHEMICAL PROPERTIES

Basaltic rocks have different geomechanical properties such as mineral content, chemical composition, and geomechanical properties. Therefore, in this study samples were collected different regions of Turkey (Fig. 1). In addition to the samples collected, some samples are adapted from the study of Korkanç (2003) in order to provide sample variety. After collection of basaltic rock samples, petrographic studies were carried out. Thin sections were prepared to determine the petrographic characteristics of different types of basalt samples. Detailed investigations on thin section samples were examined and photographed by using the LEICA DM 750 P model polarizing microscope in the context of petrographical observations. With the help of abovementioned petrographical analysis, microstructures of each sample were defined. Corresponding description of thin sections, petrographic classification, rock name, mineral content and texture are tabulated in Table 2. In this context, mineralogical – petrographical analysis along with the chemical composition determination of the basaltic rocks samples were performed. In order to specify physical and mechanical properties of rocks; water absorption, unit weight, porosity, uniaxial compressive strength (UCS), Brazilian tensile strength (BTS), point load strength (Is_{50}) and P wave velocity experiments were carried out. The corresponding sample number, location, mineral content, texture,

Table 2 Sample no, location and petrographic classification of the studied samples.

Sample No	Sampling location	Mineral content and matrix	Texture	Weathering/Hydrothermal Alteration	Rock Name	Thin section image
BS-1	Tekirdağ	Ol, Px, Pl, OM, Gl	Hypocrystalline porphyritic	Chloritization, silification	Olivine Basalt	
BS-2	Tekirdağ	Ol, Px, Pl, OM	Hypocrystalline porphyritic	Sericitization	Olivine Basalt	
BS-3	Tekirdağ	Px, Pl, Ol, OM	Holocrystalline porphyritic	Silification	Basalt	
BS-4	Niğde	CPx, Ol, Pl	Pyrotactitic	-	Basalt	
BS-5	Niğde	Ol, Px, Pl, OM	Pyrotactitic	-	Basalt	
BS-6	Niğde	Microclitic Pl, Ol, OM	Pyrotactitic	-	Olivine Basalt	
BS-7	Kütahya	Px, Pl, Ol, OM	Holocrystalline porphyritic	Carbonation, argillisation, talkization	Olivine Basalt	

BS-8	Niğde	Ol, Px, Pl, OM	Porphyritic and Pyrotaxitic	-	Olivine Basalt	
BS-9	Niğde	GL, Pl, Ol	Hyaloporphyritic and Pyrotaxitic	-	Basaltic andesite	
BS-10	İstanbul	Pl, Px, OM	Granular	Silification, sericitization	Diorite	
BS-11	Niğde	Pl, GL, Ol, Px, OM	Poicyllitic and intersertal	-	Olivine Basalt	
BS-12	Edirne	Px, Pl, Bt, Ol, OM	Holocrystalline porphyritic	Chloritization	Dyke	
BS-13	Niğde	Ol, Px, OM, Gl	Pilotaksitik	-	Olivine Basalt	
BS-14	Diyarbakır	Pl, Ol, Px, OM, Gl	Holocrystalline porphyritic	Silification, talkization	Basalt	
BS-15	Düzce	Pl, Px, OM, Gl	Holohyalin porfirik	Hydrothermal alteration	Basalt	

BS-16	Çanakkale	Pl, Px, OM, Gl	Hypocrystalline porphyritic	Hydrothermal alteration, carbonation	Basalt	
BS-17	Bolu	Pl, Px, OM, Gl	Hypocrystalline porphyritic	Hydrothermal alteration	Basalt	
BS-18	Bursa	Pl, Px, OM, Gl	Hypocrystalline porphyritic	Hydrothermal alteration	Basalt	
BS-19	Bursa	Pl, Px, Ol, OM, Gl	Hypocrystalline porphyritic	Hydrothermal alteration, carbonation	Basalt	
BS-20	Niğde	Ol, Px, Pl	Pyrotacitic	-	Olivine Basalt	
BS-21	Niğde	Px, Pl, OM	Pyrotacitic	-	Olivine Basalt	
BS-22	Niğde	Pl, Gl, Ol	Pyrotacitic	-	Basalt	
BS-23	Niğde	Px, Ol, Pl, Gl	Pyrotacitic	-	Basalt	

(CPx: Clinopyroxene, Px: Pyroxene, Pl: Plagioclase, Ol: Olivine, OM: Opaque Mineral, Bt: Biotite, Volcanic Glass: Gl)
* According to LeBas et al. (1991).

Table 3 Chemical and mineralogical properties of rock samples.

Sample No	SiO ₂ (%)	Al ₂ O ₃ (%)	Fe ₂ O ₃ (%)	CaO (%)	MgO (%)	K ₂ O (%)	Na ₂ O (%)	LOI (%)	Px (%)	Pl (%)	OI (%)	OM (%)	Matrix (%)
BS-1	46.14	13.42	10.33	8.89	12.18	2.12	3.15	1.02	9.8	7.8	18.2	2.3	64.9
BS-2	45.22	13.17	10.32	9.44	12.36	1.72	3.17	3.66	9.1	13.1	19.5	0.6	60.7
BS-3	46.17	13.71	10.44	9.63	11.52	2.44	4.88	0.71	9.3	2.1	14.2	0.4	74.4
BS-4	60.92	16.99	5.50	7.12	2.67	1.21	3.3	1.2	9	0	4.1	2	85.6
BS-5	60.97	17.01	5.48	7.09	2.96	1.21	3.27	0.6	8.2	1.5	3.1	4.6	82.8
BS-6	49.06	16.65	9.16	9.52	7.96	0.89	3.41	0.8	10.7	0	12.2	4.9	73.2
BS-7	52.01	18.12	6.05	9.02	3.14	2.51	3.43	5.78	18.7	6.3	8.5	0.7	67.8
BS-8	48.36	16.05	8.49	10.69	9.12	0.67	3.34	1	10.3	0	10.1	6	74.3
BS-9	54.36	20.37	7.04	8.96	2.67	1.81	2.97	0.6	0.3	40.1	7.8	1.3	51.2
BS-10	60.15	15.13	5.49	4.98	2.86	0.56	2.97	4.5	17.6	22.2	-	1.5	58.7
BS-11	49.69	15.48	8.21	11.02	9.14	1	3.05	0.4	7.8	1	15.8	3.8	73.6
BS-12	49.72	12.21	7.94	7.88	12.71	5.63	2.58	1.72	26	12.4	12.1	0.4	49.1
BS-13	49.17	15.91	8.74	10.7	9.31	0.68	3.33	0.1	9.5	0	16.8	5.8	68.3
BS-14	47.56	14.6	14.31	10.97	6.45	1.18	3.04	0.36	10.9	22.2	14.8	0.2	51.8
BS-15	58.12	18.16	6.72	6.51	3.14	1.28	3.49	1.05	13.4	27.3	-	0.8	58.5
BS-16	56.52	18.43	6.09	6.78	3.27	3.33	2.97	1.98	12.7	31.1	-	0.3	57.1
BS-17	58.7	16.99	7.91	5.64	3.26	1.7	4.16	1.59	16.4	27.9	-	1.3	54.2
BS-18	56.98	17.82	7.05	6.89	3.88	2.02	3.41	1.52	22.3	28.1	-	0.5	48.8
BS-19	54.28	19.93	8.96	7.70	2.76	1.12	3.51	2.09	4.8	30.8	7.4	1.1	57.6
BS-20	51.19	17.14	8.13	9.12	6.5	1.61	3.83	0.1	5.8	0.8	8	3.5	82
BS-21	51.07	17.02	8.06	9.08	6.61	1.63	3.83	0.2	10.9	2.3	10.8	5.6	72.1
BS-22	60.89	16.9	5.41	7.17	3.23	1.28	3.23	0.7	0.1	0	5.2	2.8	90.4
BS-23	60.97	16.79	5.40	7.29	2.98	1.18	3.26	0.9	7.6	0	2.6	1.8	88.4

(LOI: Loss on ignition, Px: Pyroxene, Pl: Plagioclase, Ol: Olivine, OM: Opaque Mineral)

rock name and thin section images of the samples investigated in this study are tabulated in Table 2 (Thin section images of the samples from Niğde region, i.e. samples of BS 4-5-6-8-9-10-11-13-20-21-22-23 are adapted from the studies of Korkanç (2003), Korkanç and Tuğrul (2004), Korkanç and Tuğrul (2005). In terms of the classification of basaltic rocks, LeBas et al. (1991) classification system was taken into consideration. According to the system abovementioned, rock samples were described as olivine basalt, basalt, basaltic andesite, diorite and dyke. The most common minerals are olivine, pyroxene, plagioclase, opaque mineral and volcanic glass. Right after the completion of petrographic studies, major element oxides analysis was calculated on the granulated rock samples and obtained results are provided in Table 3. In terms of chemical analysis, the ICP (Inductively Coupled Plasma) method was applied to powder samples (<200 µm) (obtained by grinding at laboratory scale mills) to determine the major element oxides contents of basalts with different facies.

Basalt materials are classified according to their SiO₂ content as alkaline basalts (up to 42 % SiO₂), mildly acidic basalts (43 to 46 % SiO₂) and acidic basalts (over 46 % SiO₂). Basaltic rocks which has SiO₂ content above 46 % (acidic basalt) with constant composition show ability to melt without solid residue, appropriate melt viscosity for fiber formation and ability to solidify in a glassy phase without noticeable crystallization (Park et al., 1999; Militky

and Kovacic, 1996). Almost all samples studied have volcanic glass in their mineral content and matrix. Studied basaltic rocks samples consist of pyroxene, plagioclase and olivine minerals. As shown in Table 3, SiO₂ contents of studied samples are in a range between 45.22 % and 60.97 %.

2.2. PHYSICAL AND MECHANICAL PROPERTIES

Block samples with an average size of 30x30x30 cm were collected to be used in laboratory experiments. Determination of physical properties (unit weight, porosity, water absorption and P wave velocity) for each corresponding basalt sample were carried out according to the method suggested by ISRM (2007). The mechanical properties of the samples were determined according to ASTM (2001a, 2010). Mechanical properties such as uniaxial compressive strength (UCS), Brazilian tensile strength (BTS) and Point load strength index (I_{s50}) were determined on the core samples. In terms of physical properties determination, cylindrical core samples with a length / diameter ratio between 2.0-2.5 were prepared in accordance with ISRM (2007) standards. Experiments were performed on basalt samples with 54.7 mm (NX), 42 mm (BX) and 36.5 mm (BQ) diameter. In this context, a total of 23 samples were tested and each test results are tabulated in Table 4. The physical and mechanical properties of the studied basaltic rocks were also compared with each other (Figs. 2 and 3).

Table 3 Chemical and mineralogical properties of rock samples.

Sample No	SiO ₂ (%)	Al ₂ O ₃ (%)	Fe ₂ O ₃ (%)	CaO (%)	MgO (%)	K ₂ O (%)	Na ₂ O (%)	LOI (%)	Px (%)	Pl (%)	OI (%)	OM (%)	Matrix (%)
BS-1	46.14	13.42	10.33	8.89	12.18	2.12	3.15	1.02	9.8	7.8	18.2	2.3	64.9
BS-2	45.22	13.17	10.32	9.44	12.36	1.72	3.17	3.66	9.1	13.1	19.5	0.6	60.7
BS-3	46.17	13.71	10.44	9.63	11.52	2.44	4.88	0.71	9.3	2.1	14.2	0.4	74.4
BS-4	60.92	16.99	5.50	7.12	2.67	1.21	3.3	1.2	9	0	4.1	2	85.6
BS-5	60.97	17.01	5.48	7.09	2.96	1.21	3.27	0.6	8.2	1.5	3.1	4.6	82.8
BS-6	49.06	16.65	9.16	9.52	7.96	0.89	3.41	0.8	10.7	0	12.2	4.9	73.2
BS-7	52.01	18.12	6.05	9.02	3.14	2.51	3.43	5.78	18.7	6.3	8.5	0.7	67.8
BS-8	48.36	16.05	8.49	10.69	9.12	0.67	3.34	1	10.3	0	10.1	6	74.3
BS-9	54.36	20.37	7.04	8.96	2.67	1.81	2.97	0.6	0.3	40.1	7.8	1.3	51.2
BS-10	60.15	15.13	5.49	4.98	2.86	0.56	2.97	4.5	17.6	22.2	-	1.5	58.7
BS-11	49.69	15.48	8.21	11.02	9.14	1	3.05	0.4	7.8	1	15.8	3.8	73.6
BS-12	49.72	12.21	7.94	7.88	12.71	5.63	2.58	1.72	26	12.4	12.1	0.4	49.1
BS-13	49.17	15.91	8.74	10.7	9.31	0.68	3.33	0.1	9.5	0	16.8	5.8	68.3
BS-14	47.56	14.6	14.31	10.97	6.45	1.18	3.04	0.36	10.9	22.2	14.8	0.2	51.8
BS-15	58.12	18.16	6.72	6.51	3.14	1.28	3.49	1.05	13.4	27.3	-	0.8	58.5
BS-16	56.52	18.43	6.09	6.78	3.27	3.33	2.97	1.98	12.7	31.1	-	0.3	57.1
BS-17	58.7	16.99	7.91	5.64	3.26	1.7	4.16	1.59	16.4	27.9	-	1.3	54.2
BS-18	56.98	17.82	7.05	6.89	3.88	2.02	3.41	1.52	22.3	28.1	-	0.5	48.8
BS-19	54.28	19.93	8.96	7.7	2.76	1.12	3.51	2.09	4.8	30.8	7.4	1.1	57.6
BS-20	51.19	17.14	8.13	9.12	6.5	1.61	3.83	0.1	5.8	0.8	8	3.5	82
BS-21	51.07	17.02	8.06	9.08	6.61	1.63	3.83	0.2	10.9	2.3	10.8	5.6	72.1
BS-22	60.89	16.9	5.41	7.17	3.23	1.28	3.23	0.7	0.1	0	5.2	2.8	90.4
BS-23	60.97	16.79	5.40	7.29	2.98	1.18	3.26	0.9	7.6	0	2.6	1.8	88.4

Table 4 Geomechanical properties of rock samples.

Sample No	Unit weight (kN/m ³)	Porosity (%)	Water abs. (%)	P-wave velocity (m/sec)	Point load strength index (MPa)	Uniaxial compressive strength (MPa)	Brazilian tensile strength (MPa)
BS-1	28.10	1.81	0.72	6988	19.88	256.00	29.00
BS-2	28.78	0.94	0.26	6974	18.24	277.00	26.00
BS-3	28.53	0.55	0.21	6523	18.01	241.00	33.00
BS-4	25.48	5.63	0.93	4959	7.11	166.70	17.00
BS-5	25.55	6.34	0.82	4907	8.90	175.50	17.00
BS-6	26.64	8.01	0.90	5873	16.56	188.80	24.00
BS-7	27.18	1.27	0.49	5627	12.89	186.00	24.00
BS-8	27.18	3.95	0.82	6175	17.91	195.30	29.00
BS-9	27.52	2.63	0.57	5401	11.19	173.90	21.00
BS-10	26.40	0.80	0.45	5236	7.85	191.00	20.00
BS-11	28.34	2.40	0.62	5817	15.37	177.90	23.00
BS-12	27.77	1.88	0.88	5867	14.82	175.00	19.00
BS-13	27.51	9.75	0.71	5957	16.67	232.30	27.00
BS-14	28.51	0.83	0.64	6328	18.74	268.00	26.00
BS-15	27.14	0.21	0.13	5306	8.23	190.00	21.00
BS-16	26.30	0.40	0.16	5502	9.99	188.00	18.00
BS-17	26.68	0.71	0.49	5368	7.64	194.00	19.00
BS-18	26.65	0.42	0.21	5564	7.68	173.00	20.00
BS-19	26.63	1.28	0.61	5428	10.52	186.00	19.00
BS-20	26.16	10.01	0.80	5694	13.93	182.60	19.00
BS-21	26.87	6.41	0.82	5718	14.42	167.50	22.00
BS-22	25.21	6.33	1.16	4737	7.10	165.30	19.00
BS-23	25.83	3.68	0.71	5219	7.22	136.10	16.00

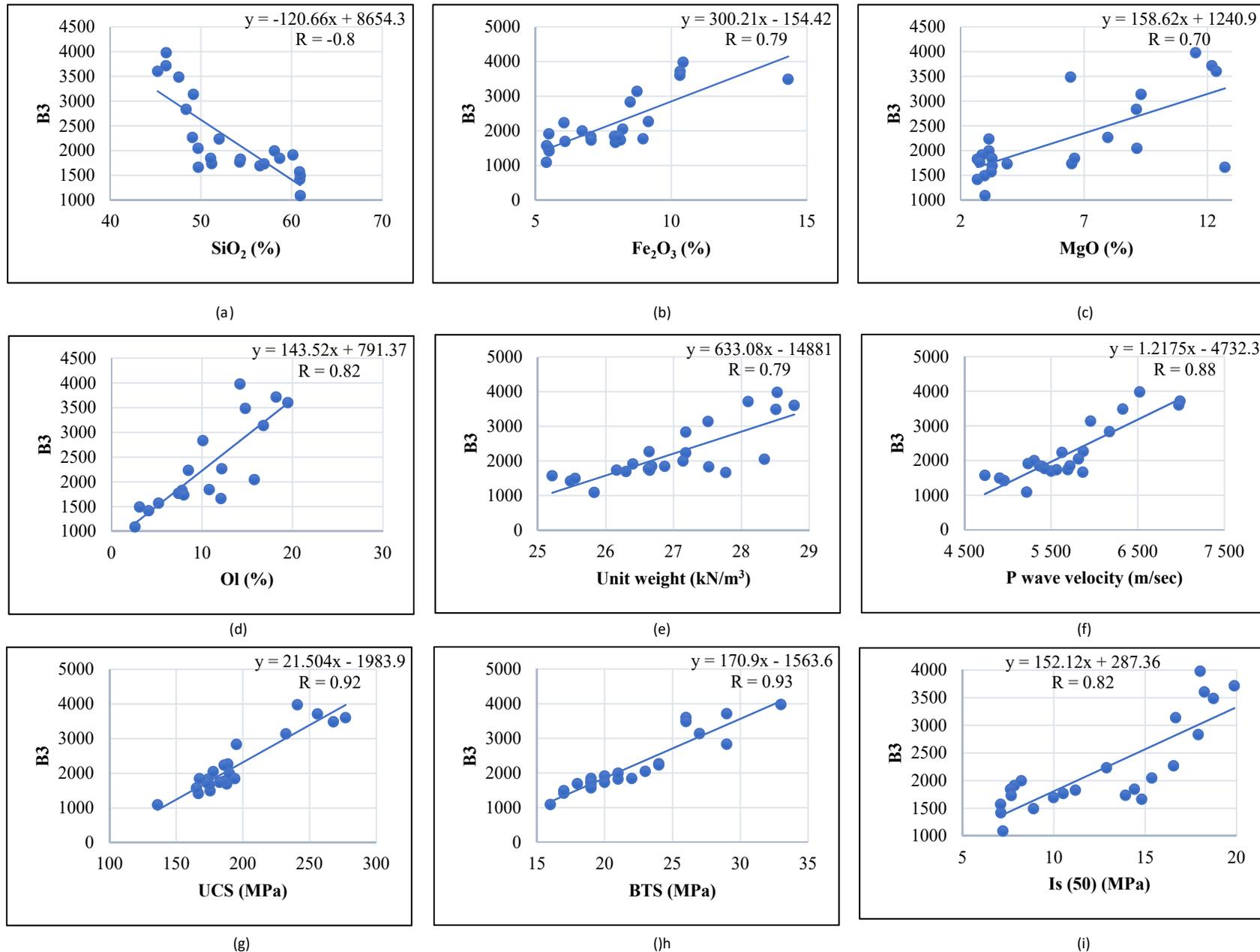


Fig. 2 Correlation between brittleness indices (B3) and SiO₂ (a) - Fe₂O₃ (b) - MgO (c) – olivine (d) - unit weight (e) – P wave velocity (f) – UCS (g) – BTS (h) – Is (50) (i) content.

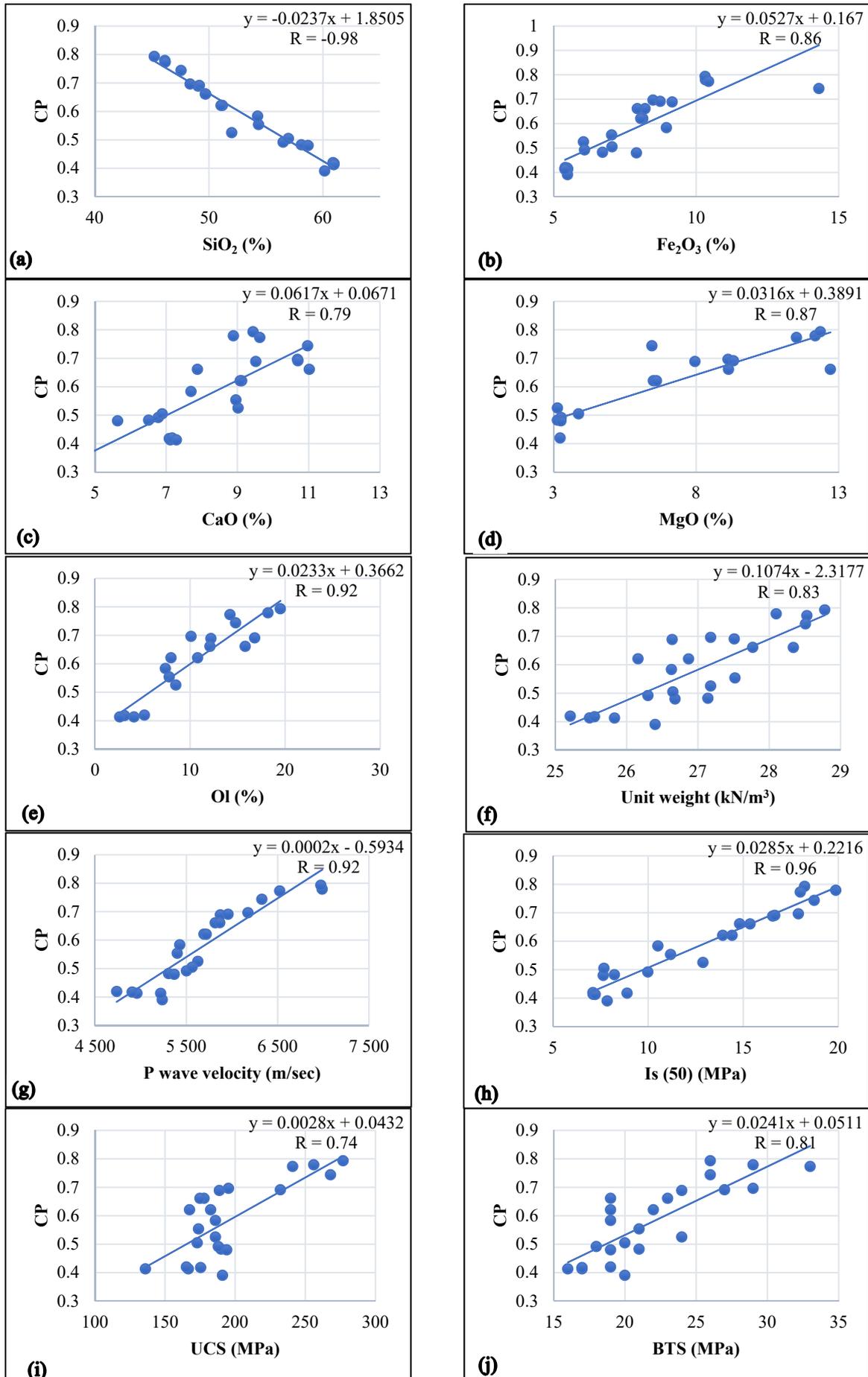


Fig. 3 Correlation between chemical parameter (CP) and SiO₂ (a) – Fe₂O₃ (b) – CaO (c) – MgO (d) olivine (e) – unit weight (f) – P wave velocity (g) – Is (50) (h) – UCS (i) – BTS (j).

Table 5 Calculated brittleness values and CP of rocks.

Sample No	B1	B2	B3	CP
BS-1	8.83	0.80	3712	0.78
BS-2	10.65	0.83	3601	0.79
BS-3	7.30	0.76	3976.5	0.77
BS-4	9.81	0.81	1416.95	0.41
BS-5	10.32	0.82	1491.75	0.42
BS-6	7.87	0.77	2265.6	0.69
BS-7	7.75	0.77	2232	0.53
BS-8	6.73	0.74	2831.85	0.70
BS-9	8.28	0.78	1825.95	0.55
BS-10	9.55	0.81	1910	0.39
BS-11	7.73	0.77	2045.85	0.66
BS-12	9.21	0.80	1662.5	0.66
BS-13	8.60	0.79	3136.05	0.69
BS-14	10.31	0.82	3484	0.74
BS-15	9.05	0.80	1995	0.48
BS-16	10.44	0.83	1692	0.49
BS-17	10.21	0.82	1843	0.48
BS-18	8.65	0.79	1730	0.50
BS-19	9.79	0.81	1767	0.58
BS-20	9.61	0.81	1734.7	0.62
BS-21	7.61	0.77	1842.5	0.62
BS-22	8.70	0.79	1570.35	0.42
BS-23	8.51	0.79	1088.8	0.41

On the other hand for basaltic rocks, if volcanic glass and plagioclase is predominant in matrix, rock is being defeated at lower strengths (Erişiş, 2016). For example, BS-2 has the lowest SiO₂ value (45.22 %) and the highest UCS value (277 MPa), on the contrary BS-23 has the highest SiO₂ value (60.97 %) and the lowest UCS value (136.3 MPa).

3. RESULTS AND DISCUSSION

The following three definitions (Eqns. (1), (2) and (3)) are widely used in previous studies to quantify brittleness indirectly based on strength:

$$B1 = \frac{\sigma_c}{\sigma_t} \quad (1)$$

$$B2 = \frac{\sigma_c - \sigma_t}{\sigma_c + \sigma_t} \quad (2)$$

$$B3 = \frac{\sigma_c * \sigma_t}{2} \quad (3)$$

where B1, B2, B3 is brittleness index (suggested by Hucka and Das 1974 and Altındag 2002), σ_c is the uniaxial compressive strength and σ_t is Brazilian tensile strength. These formulas have been employed in many studies. Uniaxial compressive strength and Brazilian tensile strength are basic rock mechanical properties and are respectively easy to obtain. And these above-mentioned properties can be employed to predict brittleness index when other data are unavailable (Table 5).

Brittle behavior of rocks is related to mineral composition. It is generally assumed that brittle rocks have a high content of brittle minerals which causes an increase in the brittleness. The presence of such

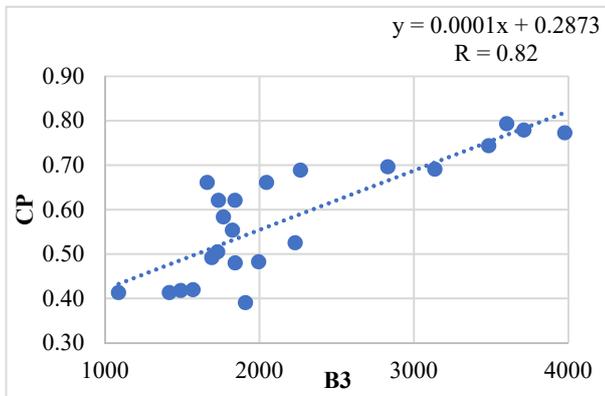
minerals (e.g., quartz, dolomite) makes rocks more brittle whereas the presence of clay minerals makes rocks more ductile. As clay content increases, the content of brittle mineralogy along with the geomechanical brittleness indices decreases accordingly. This difference may be the result of the fact that geomechanical properties of rocks are the complex function of both mineralogy and microstructure (Guo et al., 2013). In addition to this, rock properties such as chemical composition can also be used for the quantification of rock brittleness of basaltic rocks.

In this study a chemical parameter (CP) identified considering major oxides of basalt samples. Al₂O₃, Fe₂O₃ and MgO values (show positive linear correlation with B3, Fig. 2) were chosen for the nominator of the formula, while SiO₂ values (show negative linear correlation with B3, Fig. 3) were chosen for the denominator (Eqn. (4)). The mineralogical composition of the basalts used in this study is olivine, pyroxene, plagioclase and volcanic glass. Olivine, pyroxene and plagioclase minerals have high Al₂O₃, Fe₂O₃ and MgO content respectively while volcanic glass has high SiO₂ content. As regards to new formulization proposed (Eqn. (4)), chemical parameter is decreasing with high SiO₂ content which means more brittle behavior while it is increasing with Al₂O₃, Fe₂O₃ and MgO which means more ductile material.

This new chemical parameter (CP) which was proposed for predicting brittleness via major oxide element components of basaltic rocks is presented in Eqn. (4).

Table 6 Correlation coefficients (R values) for Brittle index, CP with chemical content, mineral content, physical properties and mechanical properties.

Chemical Properties	B1 R value	B2 R value	B3 R value	CP R value
SiO ₂	0.30	0.34	-0.80	-0.98
Al ₂ O ₃	0.00	2E-05	0.57	0.55
Fe ₂ O ₃	0.00	0.01	0.79	0.86
CaO	0.44	0.48	0.60	0.79
MgO	0.24	0.27	0.70	0.87
K ₂ O	0.10	0.11	0.05	0.13
Na ₂ O	0.24	0.28	0.24	0.13
Ignition Loss	0.15	0.14	3E-05	0.19
Chemical Properties	B1 R value	B2 R value	B3 R value	CP R value
Pyroxene	-	0.05	0.03	0.03
Plagioclase	0.41	0.41	0.12	0.18
Olivine	0.00	0.07	0.82	0.92
Opaque Mineral	0.43	0.44	0.03	0.12
Matrix	0.26	0.26	0.18	0.21
Physical Properties	B1 R value	B2 R value	B3 R value	CP R value
Unit Weight	0.17	0.20	0.79	0.83
Porosity	0.16	0.14	0.18	4E-05
Water Absorbtion	0.16	0.14	0.26	0.05
P Wave Velocity	0.14	0.18	0.88	0.92
Mechanical Properties	B1 R value	B2 R value	B3 R value	CP R value
Point load strength	0.30	0.33	0.82	0.96
Uniaxial compressive strength	0.21	0.17	0.92	0.74
Brazilian tensile strength	0.47	0.51	0.93	0.81

**Fig. 4** Correlation between brittleness indices B3-CP.

$$CP = \frac{Al_2O_3 + Fe_2O_3 + MgO}{SiO_2} \quad (4)$$

Simple regression analysis was used to obtain relationships between brittleness index and geological properties of basaltic rocks. The correlation coefficients (R) were calculated by the least square method and linear functions were considered during these tests (Figs. 2, 3, 4). Since there are no strong correlation coefficients between B1 and B2 and rock properties, the results of regression analysis indicated that the B1 and B2 are not so meaningful for the understanding of the brittle or ductile behavior of the samples investigated.

On the other hand there is a strong relationship, including high correlation coefficient, between the rock properties and B3. For instance, Kaunda and Asbury (2016) determined brittleness using artificial neural networks (ANN) on hard rocks and they found that there were meaningful relationship between P-S wave and elastic properties. In this study S wave and elastic properties were not studied but the other highest correlation coefficients were calculated with P-wave velocity as 0.88 for B3 (Figs. 2f and 3e).

As a result, linear relationships were obtained between CP and Fe₂O₃, CaO, MgO, olivine, unit weight, P wave velocity, Is (50) point load strength, uniaxial compressive strength and Brazilian tensile strength, respectively. However, only negative linear relation exists between CP and SiO₂ content (Fig. 3).

As regards to Figures 3 (a-j), chemical parameter (CP) has significant relations with various parameters analyzed. R values are tabulated in Table 6. Accordingly, R-values (for each parameter with respect to B1, B2, B3 and CP) are provided in Table 6; respectively for chemical content, mineral content, physical properties and mechanical properties.

Referring to Table 5, highest R values (represented in bold) are denoted as very significant and very strong relation. For example, highest R value is obtained from the plot of SiO₂ content and CP (negative R value, -0.98). On the other hand, lowest R values are obtained with "ignition loss" parameter. In this case in terms of brittleness index, SiO₂ content should be considered in detail, while it was found out ignition loss is no effective parameter on brittleness.

Olivine content is also observed to be the one showing significant relationship among others and it resulted as highest R values such as 0.82 and 0.92 respectively for B3 and CP. As in the same context, Table 6 summarizes the fact that unit weight among physical properties has the highest R values compared to other physical properties. This abovementioned explanation is only valid for B3 and CP values, and the corresponding R values are 0.79 and 0.83, respectively. In terms of mechanical properties, point load strength, uniaxial compressive strength and Brazilian tensile strength with respect to B3 and CP has very significant R values (0.93 for B3 and 0.96 for CP, respectively). In order to represent the correlation between brittleness indices, Figure 4 is provided.

As regards to Figure 4, correlation between B3 and CP yields a R value of 0.82. It is not always possible to evaluate mechanical or physical properties of rocks and that is why it would not be possible to understand brittleness behavior of rocks based on the definitions of B3 (Altindag, 2002) nor based on the brittleness index approaches given in Table 1. Because of this abovementioned reason, CP can be proposed to be an alternative way to understand brittleness behavior of rocks. Altindag (2010) has also evaluated brittleness indexes in rock-drilling efficiency, and he pointed out the fact that "there is no standardized and universally accepted brittleness concept". In the same study of Altindag (2010), author has not only pointed out the abovementioned fact but also he stated the fact that there is no available measuring method of rock brittleness. Referring back and forth to the study of Altindag (2010), many researchers (Baron, 1962; Coates and Parsons, 1966; Lawn and Marshall, 1979; Aubertin et al., 1994; Quinn and Quinn, 1997; Ribacchi, 2000; Hajiabdolmajid and Kaiser, 2003) have either introduced brittleness index obtained from stress-strain curves or employed these indexes accordingly. Altindag (2010) has pointed out Andreev (1995) review which has 20 different formulations and definitions of rock brittleness. Although as summarized by Altindag (2010), still no available brittleness index of rocks widely accepted, CP in this context can be proposed to be a new and novel approach. CP can be employed well in terms of brittleness characterization of rocks and corresponding validation of it is provided either by tabulated R values or the corresponding graphical demonstrations (Figure 3 and Figure 4). And in this context, for basalt type rocks in specific, this new approach for brittleness index prediction (CP) is claimed to be better successful than any other approaches proposed previously.

Rock brittleness as it is described by Zhang et al. (2021) is affected by internal and external factors and mineral composition of rocks is one of the primary internal factor affecting the rock brittleness. Mineral composition of rocks changes with their chemical composition so does the rock brittleness. Although brittleness is mostly evaluated based on stress-strain curve, special tests, mineral composition & porosity,

Mohr Envelope (Zhang et al., 2021), differentiation in brittleness index values could have been related to differentiation of chemical composition for the specific rock samples, i.e. basalt samples in this case. Meng et al. (2021) have reviewed 11 groups of brittleness indices in their study and chemical composition or chemical parameter was not even partly considered. Not only brittleness indices based on mineral contents (B₄₈ to B₅₄ from the study of Meng et al., 2021) can be regarded as ignoring the important factors (stress state, diagenesis, pre-consolidation pressure, porosity, grain size, type and strength of cementing material) contributing the rock brittleness (Meng et al., 2021), but also it can be claimed that they (brittleness indices based on mineral contents) overlay the importance of chemical composition in this regard. Mineral and chemical composition of basalts are generally questioned together as in the study of Aal (1998) and Babievskaya et al. (2009) have proposed a method to calculate mineral composition of basaltic rocks based on their chemical composition. So, referring abovementioned researchers (Aal, 1998 and Babievskaya et al., 2009) once again, chemical composition of basaltic rocks should be as significant as mineral composition in terms of brittleness behavior evaluation.

4. CONCLUSION

In this study, suggested CP formula provided very high strength correlation coefficients (Table 5) accordance with the SiO₂ (R: -0.98), olivine (R: 0.92), P wave velocity (R: 0.92), Is₍₅₀₎ (R: 0.96), in addition to it has high strength correlation coefficients accordance with Fe₂O₃ (R: 0.86), CaO (R: 0.79), MgO (R: 0.87), unit weight (R: 0.83), UCS (R: 0.74), BTS (R: 0.81) (Table 5). By taking these strong correlation coefficients into account, it is thought that the newly suggested chemical parameter (CP) formula will be useful to determination and prediction of rock brittleness if chemical content of rock is known. In this way different properties of basaltic rocks could be predicted with the help of this formula (CP). The traditional methods for strength measurement require core samples, which are expensive and are time consuming; and maintaining the in-situ condition and sample preparation are effortful circumstances. So the use of major oxide-based on CP for predicting of basaltic rocks brittleness behavior, instead of strength-based B3 (R=0.82) brittleness index is highly reliable.

It is concluded that, B1 and B2 are not reliable parameters for predicting the different properties however, B3 and CP can be used as good criteria for predicting the different properties of basaltic rocks (especially chemical and mechanical properties).

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